

Using FEA to Predict Failure Mechanisms in RC Shear Walls



PRESENTED BY

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2 **Overall Project Contributors**

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3 Introduction

Project goal: to reduce the field construction times and fabrication costs of RC nuclear structures

- High-strength steel rebar
- Prefabricated rebar assemblies with headed anchorages
- High-strength concrete

FEA goal: to create a validated numerical model that can extrapolate experimental results to geometries not tested in the lab

- Predict total strength
- Predict lateral displacement capacity
- Predict initial lateral stiffness
- Predict crack patterns and damage propagation
- Determine ultimate failure mechanism

4 Wall Geometry



5 Rebar Layout



Single layer of overlapping bars on each face at 6" o.c. No. 6 bars for normal-strength steel, No. 4 bars for high-strength steel. Additional trim reinforcement around penetrations.

⁶ FEA Geometry and Mesh

Half-symmetry simulation

0.5" mesh element size on the shear wall



Wall cap and base were modeled as elastic while the shear wall could accrue damage

Concrete was modeled with a total of 192,222 8 -noded hexahedral elements



7 Boundary Conditions and Load Application



Base nodes fixed in all DOF

⁸ FEA Rebar Geometry

0.5" mesh elements on all rebar

Rebar was modeled with a total of 5,774 2-noded beam elements

Beam elements were fully embedded in the concrete with no slip and extruded to the proper diameter (No. 6, No. 4, etc.)



9 Loading Protocol

Elastic regime

Damaging regime



10 Material Model Assumptions

Rebar: modeled as bi-linear elastic plastic



Concrete: modeled as either pure elastic or Holmquist Johnson Cook concrete

• Normal strength f'c = 7 ksi, high strength = 15 ksi

- Shear wall base and cap (foundation and loading block) were modeled as pure elastic
- Shear wall center portion modeled with Holmquist Johnson Cook concrete model
 - Accrues damage and is designed to remain stable in all FEA simulations
 - Strength can increase with confining pressures
 - All variables in the model are based on experiment as dictated in the original paper



12 Normal-Strength Concrete and Rebar



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¹³ Normal-Strength Wall Cracking and DIC Comparison



3D DIC is a non-contact near-full-field optical measurement technique that calculates surface deformations within a specified field-of-view on the test specimen .

14 Normal-Strength Concrete and Rebar Force Results



15 High-Strength Concrete and Rebar



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¹⁶ High-Strength Wall Cracking and DIC Comparison



3D DIC is a non-contact near-full-field optical measurement technique that calculates surface deformations within a specified field-of-view on the test specimen.

17 High-Strength Concrete and Rebar Force Results



¹⁸ Final Failure Mechanism (both walls)



Wall final failure mechanism was shear slipping along the base along with extensive concrete cracking and spalling.

19 **Conclusions**

The FEA simulation was able to capture the behavior of the shear wall including damage propagation and final failure mechanism.

Both FEA simulations (NS and HS) show wall cracking patterns similar to observed in experiment.

Normal-strength material wall conclusions:

- Accurately predicted lateral stiffness
- Predicted a lower peak strength than was seen in experiment

High-strength material wall conclusions:

- Accurately predicted peak strength
- Accurately predicted initial undamaged lateral stiffness
- Predicted a higher lateral stiffness between undamaged and fully damaged than was seen in experiment

20 **References**

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